

**DESIGN CALCULATION AND DRAWING FOR
FIXED BEARING
(Ch. 516+938)**

PROJECT: FOUR LANING OF JHANJHI TO DEMOW SECTION OF NH-37 FROM EXISTING CH. K 491+050 TO KM 535+250
(DESIGN CH. KM 4900+800 TO KM 534+800) IN THE STATE OF ASSAM UNDER EPC MOD

CLIENT: *NATIONAL HIGHWAYS & INFRASTRUCTURE DEVELOPMENT CORPORATION LTD. (NHIDCL)*

CONTRACTOR : *M/S KAMAC-SHIVA HARLALKA (JV)*

MANUFACTURER:



M/S KARMA ENTERPRISE, GUWAHATI, ASSAM

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Fixed Bearing

TYPE OF BEARING : FIX
REVISION : 00

TYPE: B1
CH: 516+938

Conc. Grade for Pedestal M 45
Conc. Grade for Superstructure M 50

DESIGN DATA FOR BEARING DESIGN (LOADS)

LOADING PARAMETERS	SLS			ULS	
UNITS	MT	kN		MT	kN
VERTICAL MAXIMUM LOAD	167.94	1647.00		241.16	2365.00
VERTICAL MINIMUM LOAD	68.63	673.00		117.27	1150.00
VERTICAL PERMANENT LOAD	85.96	843.00			
HORIZONTAL LOADS					
ACTING IN LONGITUDINAL DIRECTION	5.10	50.00		7.65	75.00
ACTING IN TRANSVERSE DIRECTION	0.00	0.00		0.00	0.00
RESULTANT HORIZONTAL LOADS		50.00			75.00
DISPLACEMENT	0.00			0.00	
LONGITUDINAL (MM)	0.00			0.00	
TRANSVERSE (MM)	0.00			0.00	
ROTATION (RADIAN)	0.0038	+	0	=	0.004

Assume Permanent Rotation q p = 0.41 x 0.004 = 0.002
Assume Variable Rotation q v = 0.59 x 0.004 = 0.002

MATERIALS

Steel Stress (Working) for Design Use 340 MPa (Grade-340-570W) AS PER IS - 1030
HT Bolts shall conform to Grade 8.8 of IS : 1364
Elastomer shall be of hardness 50 +/- 5 conforming to IRC : 83 (Part III) - 2018 Table - 4.3

DIMENSION DETAILS OF BEARING COMPONENTS

ELASTOMERIC PAD

PAD DIAMETER = di 325 mm
PAD THICKNESS = hc 24 mm

CYLINDER

CYLINDER CONCRETE CONTACT DIAMETER = Do 395 mm
CYLINDER BASE THICKNESS = kb 20 mm
CYLINDER INNER DIAMETER = Di = di 325 mm
CYLINDER OUTER DIAMETER = do 375 mm
HEIGHT OF CYLINDER = hc 36 mm
WALL THICKNESS = Tew 25.0 mm

TOP COMPONENT

TOP COMPONENT CONCRETE CONTACT DIA. = Dpeff 375 mm
TOP COMPONENT DIA. = Dp 375 mm
TOP COMPONENT ABOVE THICKNESS = Tp 22 mm
TOP COMPONENT PROJECTION = hp 22 mm
VERTICAL FACE = w 6 mm

ANCHORAGE

BOLTS DIAMETER = Dbolt 16 mm
BOLTS LENGTH = Lbolt 35 mm
BOLTS PER COMPONENT = Nbolt 4 NOS.
GRADE OF BOLTS = GR.bolt 8.8
ANCHORAGE COLLOR LENGTH = CL 56 mm
ANCHORAGE COLLOR THICKNESS = C thk 12 mm
SLEEVE LENGTH (Superstructure) = Ls 110 mm
SLEEVE DIAMETER (Superstructure) = Ds 40 mm
SLEEVE LENGTH (Pedestal) = Lp 120 mm
SLEEVE DIAMETER (Pedestal) = Dp 40 mm

GENERAL

NO. OF BRASS SEALING RINGS = Nbr 2 NOS.
TOTAL THICKNESS OF RINGS = Tbr 4.0 mm
GAP BETWEEN CYLINDER & TOP COMPONENT = h4 10 mm

TOTAL BEARING ASSEMBLY HEIGHT = HT 88 mm

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Fixed Bearing

Calculation for Permissible Stresses in Pedestal Concrete

Bottom

Cylinder Concrete Contact Diameter =	395	mm
Loaded area ($A_{co} = p \times D_b \wedge 2/4$) =	122541.75	mm ²
Required Pedestal Size for Dispersion =	790.00	x 790 mm
Dispersed area ($A_{cl} = p \times d \wedge 2/4$) =	490167	mm ²

Top

Top Component Contact Diameter =	375	mm
Loaded area ($A_2 = p \times D_b \wedge 2/4$) =	110446.62	mm ²
Required Superstructure Size for Dispersion =	750.00	x 750 mm
Dispersed area ($A_1 = p \times d \wedge 2/4$) =	441786.47	mm ²

DESIGN CALCULATIONS :-

REF. CODE : IRC:83 (Part-III)-2018

Design of Pad (Clause - 5.2.3.2)

Effective diameter of Pad	Dpad	325 mm
Area of pad = $p \times d \wedge 2/4$	a	82991.0714 mm ²
Vertical Load	Nsd	2365.00 kN
Direct Pressure Nsd / a	pa	28.50 N/mm ²
(Nsd / a) x Ym		37.05 N/mm ²
fc,k		60.00 N/mm ²
		OK

Check Compression at edge of Neoprene Pad (Clause - 5.2.3.4)

Max. Permitted = 15 % of hc	3.60	mm
Desired Rotation	0.00380	radians
Available Rotation in Radius due to Compression of Pad	0.02215	radians
		OK
Diameter / Thickness Ratio	13.54	Maximum (Dpad/hc) 15
		OK

Check for Min. average Stress (Clause - 5.2.3.3)

Min. average stress = (Nsd min. / a)	8.11	N/mm ²
Permissible Min. average stress =	2.00	N/mm ²
		OK

Stress in bottom concrete

Bottom dispersion width	Do	395
Thickness of Bottom Plate	kb	20
Area of Bottom Dispersion = $p \times d \wedge 2/4$	a	122591.071 mm ²
Section Modulus = $p \times d \wedge 3/32$	Z	6052934.152 mm ³
Vertical Load	Nsd	2365.00 kN
Horizontal force	Vsd	75.00 kN
Moment of resistance due to rotation:		
Rotation due to dead load	θp	0.00155 radians
Live Load	θv	0.00225 radians
Ratio = di / hc		13.54
For induced moment	k1	1.97
	k2	76.38
Induced moment due to rotation Mc.d		
$d^3 \times (k1 \times qp + K2 \times qv) / 1000$	Mc.d	5997.30 kN-mm

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Fixed Bearing

Moment of resistance due to HF:			
Horizontal distance	C	162.50	mm
Resultant HF	Vsd	75.00	kN
Moment of resistance due to HF:	Mr.d	2437.50	kN-mm
$0.2 \times C \times Vsd$			
Total Movement = Me.d+Mr.d =	Mt	8434.80	kN-mm
Direct Pressure Nsd / a	pa	19.29	N/mm ²
Permissible Stress $(0.67 \times f_{ck}) / 1.5$	fcd	20.10	N/mm ²
OK			
Bending Stress Mt / Z	pb	1.39	N/mm ²
Permissible bending stress		14.85	N/mm ²
OK			
Area on Pedestal	Ac1	490167.00	mm ²
Frd = Aco x fcd x (sqrt(Ac1 / Aco))		4926.18	kN
3 x fcd x Aco		7389.27	kN
Frd <= 3 x fcd x Aco		OK	

Stress in top concrete

Top dispersion width		Dpeff	375
Thickness of Top Plate		TP	22
Area of Top Dispersion = $p \times d^{2/4}$	a	110491.071	mm ²
Section Modulus = $p \times d^{3/32}$	Z	5179268.973	mm ³
Vertical Load			
Nsd		2365.00	kN
Horizontal force			
Vsd		75.00	kN
Moment of resistance due to rotation:			
Rotation due to dead load	θ_p	0.00155	radians
Live Load	θ_v	0.00225	radians
Ratio = di / he		13.54	
For induced moment			
k1		1.97	
k2		76.38	
Induced moment due to rotation	Me.d		
$di^3 \times (k1 \times qp + K2 \times qv) / 1000$		5997.30	kN-mm
Moment of resistance due to HF:			
Horizontal distance	C	162.50	mm
Resultant HF	Vsd	75.00	kN
Moment of resistance due to HF:	Mr.d	2437.50	kN-mm
$0.2 \times C \times Vsd$			
Total Movement = Me.d+Mr.d =	Mt	8434.80	kN-mm
Direct Pressure Nsd / a	pa	21.40	N/mm ²
Permissible Stress $(0.67 \times f_{ck}) / 1.5$	fcd	22.33	N/mm ²
OK			
Bending Stress Mt / Z	pb	1.63	N/mm ²
Permissible bending stress		16.50	N/mm ²
OK			
Area on Superstructure	Ac1	441786.47	mm ²
Frd = Aco x fcd x (sqrt(Ac1 / Aco))		4933.28	kN
3 x fcd x Aco		7399.92	kN
Frd <= 3 x fcd x Aco		OK	

Pot walls subjected to tensile force (Clause 5.3.1.2.3)

AR = (do - di) x hc	AR	1800.00	mm ²
Ve,sd = 4 x Nsd x he / π x di	Ve,sd	222.48	kN
Vfxy,sd		75.00	kN
Vsd = Ve,sd + Vfxy,sd		297.48	kN
Vrd = fy x AR / Ym	Ym =	1.1	kN
OK			

Pot walls subjected to shear force (Clause 5.3.1.2.4)

V'sd $\leq V_{rd}$			
V'sd = Ve,sd + 1.5 x Vfxy,sd / di		1.03	kN
V'rd = (fy x (do - di)) / (2 x Ym x (sqrt(3)))	Ym = 1.1	5.15	kN
OK			

Pot base subjected to tensile force (Clause 5.3.1.2.5)

Vsd $\leq V_{rd}$			
Ap = do x kb		7500.00	mm ²
Vsd = Ve,sd + Vfxy,sd		297.48	kN
Vrd = fy x Ap / Ym	Ym =	1.1	kN
OK			

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Fixed Bearing

Anchor Bolts

Max Horizontal Force		75.00	kN
Min. Vertical Load		1150.00	kN
Frictional Force		0.00	kN
Diameter of Bolt		16.00	mm
Length of Bolt		35.00	mm
Number of Bolts	n	4.00	Nos.
Thickness of Collar	Ct	12.00	mm
Factor for Net Area	kn	0.78	
Effec. Area of Bolt $(p \times d^2 \times \pi / 4)$	Abolt	156.89	mm ²
σ_v		0.60	
f_{ub}		800.00	N/mm ²
Y_m		1.25	
f_u		570.00	N/mm ²
k2		0.90	
Shear resistance $F_{v,Rd} = \sigma_v \times f_{ub} \times A / Y_m$	$F_{v,Rd}$	60.25	kN
Resultant horizontal force / bolt $V_{fy,sd} / n$	$F_{v,sd}$	18.75	kN
		OK	
Bearing resistance $F_{b,Rd} = 1.25 \times f_u \times d \times t / Y_m$	$F_{b,Rd}$	109.44	kN
Resultant horizontal force / bolt $V_{fy,sd} / n$		18.75	kN
		OK	
Tension resistance $F_{t,Rd} = k2 \times f_{ub} \times A_s / Y_m$	$F_{t,Rd}$	90.37	kN
Design tension resistance	$F_{t,sd}$	4.53	kN
Combined shear and tension $= F_{v,sd} / F_{v,Rd} + F_{t,sd} / (1.4 \times F_{t,Rd})$		0.35	
		OK	

Anchor Sleeves (Clause - 5.3.6.4.2)

Top Plate			
Length of Sleeve		110.00	mm
Diameter of Sleeve		40.00	mm
Number of Sleeves		4.00	Nos.
Resistance offered by concrete $FR_{du} = 1.33 D \times L \times f_{cd} / \sqrt{3}$		75.46	kN
Design resistance in shear		18.75	kN
		OK	
Bottom Plate			
Length of Sleeve		120.00	mm
Diameter of Sleeve		40.00	mm
Number of Sleeves		4.00	Nos.
Resistance offered by concrete $FR_{du} = 1.33 D \times L \times f_{cd} / \sqrt{3}$		74.08	kN
Design resistance in shear		18.75	kN
		OK	

Requirement of clearance

$\delta = 0.01 \times D_i$	min. = 3	3.25	<	10	mm
$h_c - h_e - (w - w_e) \times 0.5 - (\theta \times 0.5 \times D_i)$		8.60			mm
δ		3.25			mm
		OK			
$f_{tp} - (h_c - h_e) - (\theta \times 0.5 \times D_p)$		9.29			mm
δ		3.25			mm
		OK			

Curved Contact Surface (Clause - 5.3.1.4.2)

$V_{sd} \leq V_{rd}$	R	=	162.50	mm
	f_u	=	570.00	N/mm ²
	θ	=	0.004	Radian
	Y_m	=	1.10	
$V_{rd} = 15 \times f_u^2 \times R \times D_i / E_s \times Y_m^2$		=	1063.6	kN
V_{sd}		=	75.00	kN
		=	OK	
$w_e = 3.04 \times (\sqrt{1.5 \times V_{sd} \times R / E_s \times D_i})$		=	0.43	mm
$w_e + \theta \times D_i$		=	1.66	mm
Provided	w	=	6.00	mm
		=	OK	





Bearing Type	Cave	Vertical Load (KN) Max.	Min.	Hor. Load (KN) Loop	Moment T : L (mm)
Fixed Bearing	SLS	1647.00	673.00	50.00	0.00
	ULS	2265.00	1150.00	75.00	0.00
					± 0 ± 0

MATERIALS

1. CONFINEMENT ELASTOMER INSIDE POT WILL HAVE FOLLOWING PROPERTIES:
 - a. HARDNESS IRHD IS 50-60 PART D 36-5
 - b. MIN. TENSILE STRENGTH MPa IS 5-10 (PART D) 15.5
2. MIN. ELONGATION AT BREAK, MAX COMPRESSION SET & ACCELERATED AGING WILL BE AS PER TABLE 1. PROPERTIES OF ELASTOMER IN IRC-36/PART D
3. ANCHORS: HT BOLT'S OR GR. 3 & STEELV MATERIAL AS PER IS 2062
4. ANCHORORIES TO IT DLS / 70%2
5. CONCRETE GRADE FOR PIEDestal IS M40.
6. CONCRETE GRADE FOR STEELSTRUCTURE IS M60.

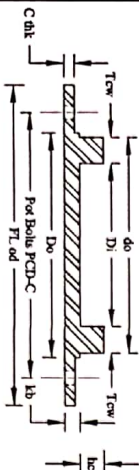
ALL WELDING WILL BE MANUAL METAL ARC PROCESS CONFORMING TO IS: 814, PRE HEATING & POST WELD STRESS RELIEVING TO BE DONE IF REQUIRED

- a) ALL NONWORKING SURFACES WILL BE COATED WITH 2 COATS OF EPOXY PRIMER & ONE OR MORE COATS EACH OF EPOXY INTERMEDIATE AND FINISH PAINT. TOTAL DRY FILM THICKNESS > 160 MICRONS.
- b) ANCHORE SLEEVES WILL BE CEMENT COATED AT SITE (IF REQUIRED).

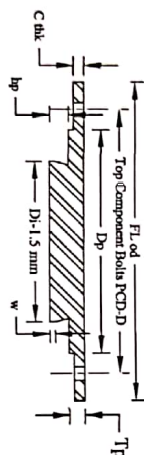
a) TESTS ON CASTINGS: TESTS SPECIFIED IN IS : 1030 WILL BE PERFORMED. CASTINGS SHALL BE ULTRASONICALLY TESTED & CERTIFICATES SUBMITTED. QUALITY LEVEL : 3 AS PER IS : 9363.

9) ALL TESTS ON BEARINGS WILL BE CARRIED OUT IN PRESENCE OF REPRESENTATIVE OF DPT/ P.M.C. NECESSARY TEST CERTIFICATES FOR RAW MATERIALS SHALL BE FURNISHED AT THE TIME OF SUPPLY.

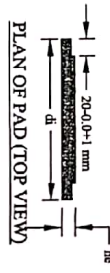
3) TOLERANCES	
a) PLAN DIMENSIONS	- 0 TO +0mm
b) OVERALL HEIGHT	- 0 TO +0mm
c) HEIGHT OF ELASTOMER	- 0 / + 5%
d) HEIGHT OF ANY STEEL COMPONENT	
1) MACHINED	- 0 TO +0mm
2) UNMACHINED	CLASS 2 OF IS : 4897



SECTIONAL ELEVATION OF BOTTOM CYLLINDER AT P-P



SECTIONAL ELEVATION OF TOP COMPONENT AT Q-Q



PLAN OF PAD (TOP VIEW)



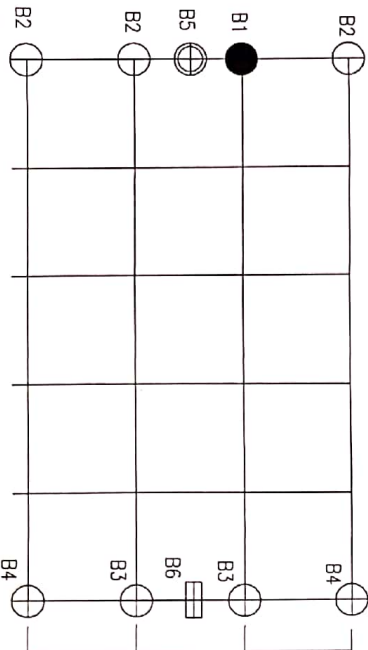
PLAN OF BRASS RING (TOP VIEW)

TYPE :- FIXED BEARING
TYPE :- B1
CH. :- 516+938

Diagram showing the coordinate system for the bridge deck. The vertical axis is labeled "TRAFFIC DIRECTION OF BRIDGE" and the horizontal axis is labeled "TRANVERSE DIRECTION OF BRIDGE".

[illegible]

BEARING TYPE & LOAD DETAILS		FIXED BEARING		LONG FIXED BEARING		TRANS FIXED BEARING		FREE BEARING		PIN BEARING		METALLIC GUIDED BEARING	
BEARING MARK		B1		B2		B3		B4		B5		B6	
TOTAL QUANTITY (NO)		1		3		1		3		1		1	
GRADE OF CONCRETE	UPPER SURFACE	M50		M50		M50		M50		M50		M50	
	ULS	M50		M50		M50		M50		M50		M50	
	SLS	M45		M45		M45		M45		M45		M45	
	ULS	M45		M45		M45		M45		M45		M45	
DESIGN LOAD (KN)	SLS	MAX		1647		1647		1647		1647		-	
		VERTICAL		843		843		843		843		-	
		PERMANENT		673		673		673		673		-	
		MIN		50		0		0		0		-	
DESIGN LOAD (KN)	ULS	0		0		0		0		0		-	
		LONGITUDINAL		2365		2365		2365		2365		-	
		VERTICAL		1150		1150		1150		1150		-	
		PERMANENT		1		1		1		1		-	
DESIGN LOAD (KN)	ULS	MIN		75		0		0		0		5548	
		LONGITUDINAL		0		0		0		0		2755	
		TRANVERSE		0		0		0		0		2755	
		LONG		-		-		-		-		-	
TRANSLATION (MM)	SLS	IRREVERSABLE		-		-		-		-		-	
		TRAN		-		-		-		-		-	
		REVERSABLE		-		14		14		-		-	
		TRAN		-		-		13		-		-	
TRANSLATION (MM)	ULS	IRREVERSABLE		-		-		-		-		-	
		TRAN		-		-		21		21		21	
		REVERSABLE		-		20		20		-		-	
		TRAN		0.00253		0.00253		0.00253		-		-	
ROTATION (RED)	SLS	IRREVERSABLE		-		-		-		-		-	
		TRAN		0.00253		0.00253		0.00253		-		-	
		REVERSABLE		-		-		-		-		-	
		TRAN		0.00380		0.00380		0.00380		0.00380		0.00380	
ROTATION (RED)	ULS	IRREVERSABLE		-		-		-		-		-	
		TRAN		0.00380		0.00380		0.00380		0.00380		0.00380	
		REVERSABLE		-		-		-		-		-	
		TRAN		0.00380		0.00380		0.00380		0.00380		0.00380	



- B1 ● FIXED BEARING
 B2 ⊖ LONG GUIDED BEARING
 B3 ⊖ LONG GUIDED BEARING
 B4 ⊕ FREE BEARING
 B5 ⊖ LONG GUIDED BEARING
 B6 ⊖ LONG GUIDED BEARING



NOTES:

- ALL DIMENSIONS ARE IN MILLIMETERS, UNLESS NOTED OTHERWISE.
- THE CONTRACTOR SHALL SUBMIT DESIGN / DRAWING OF INDIVIDUAL BEARINGS BASED ON LOAD DATA AS GIVEN IN THIS DRAWING FOR APPROVAL OF THE ENGINEER.
- BEARINGS SHALL BE PROVIDED FROM THE LIST OF APPROVED MANUFACTURERS BY THE CLIENT.
- ALL BEARINGS SHALL CONFORM TO THE LATEST INDIAN SPECIFICATION, IS: 17729-2019 AND TECHNICAL SPECIFICATION, P. 10.
- THE TESTING OF SAM SAMPLES, BEFORE COMPLETION, OF THE BEARINGS SHALL BE DONE BY THE CONTRACTOR TO THE SATISFACTION OF THE ENGINEER.
- MANUFACTURER SHALL SUBMIT THE CERTIFICATES FOR LOAD TESTING AND DIMENSIONS OF BEARING.
- SURFACE FINISH SHALL BE PROVIDED FOR PROTECTION AND WINDING ALONG WITH TENDON FOR ALIGNMENT SHALL BE PROVIDED BY THE CONTRACTOR.
- THE PLAN SIZE AND HEIGHT OF BEARING SHALL BE ADJUSTED TO SUIT THE FINISHED SIZE OF BEARINGS AT THE TIME OF EXECUTION.
- THE GROUP/BEARING MARKER SHALL BE OF HIGH STRENGTH FREE FLOWING NON-SHRINK TYPE.

KEY PLAN SHOWING ARRANGEMENT OF BEARING LOCATIONS FOR POT cum PTFE BEARINGS

CLIENT: ASB	PROJECT: FOUR LANE ROAD BRIDGE ON THE EAST SIDE OF KARMA LAKSHI	CONTRACTOR: BS	DESIGN: BS	PROJECT CONSULTANT: SKA	SAFETY CONSULTANT: SKA	AUTHORITY: MAHARASHTRA GOVT	SCALE: AS SHOWN	DATE: 10/01/2023	REVISION: 10
DESIGNER: SKA	CHECKED: SKA	APPROVED: SKA	DATE: 10/01/2023	REVISION: 10	DATE: 10/01/2023	REVISION: 10	DATE: 10/01/2023	REVISION: 10	DATE: 10/01/2023
DESIGNER: SKA	CHECKED: SKA	APPROVED: SKA	DATE: 10/01/2023	REVISION: 10	DATE: 10/01/2023	REVISION: 10	DATE: 10/01/2023	REVISION: 10	DATE: 10/01/2023
DESIGNER: SKA	CHECKED: SKA	APPROVED: SKA	DATE: 10/01/2023	REVISION: 10	DATE: 10/01/2023	REVISION: 10	DATE: 10/01/2023	REVISION: 10	DATE: 10/01/2023